

*REPORT ON*  
SOIL INVESTIGATION FOR CONSTRUCTION OF  
+2 SCHOOL AT H.S., KORARI, JODHAN BIGAHA,  
BELCHHI, PATNA

*Submitted to*

**CHIEF ENGINEER  
BSEIDC, PATNA.**

**SHAMVWI CONSULTANT**  
414, Jagat Trade Centre,  
Fraser Road, Patna – 800 001  
Tel.: 0612 – 2973107, 2366308, 2365145  
Fax: 0612 – 2214287  
Mobile: +919835218184, 8986215718.

## PREFACE

The present report on sub-soil investigation was carried out as per Chief Engineer, BSEIDC, Patna letter no BSEIDC/TECH/1960/2018-4981 dated 03.09.2019.

The entire investigation process was broadly divided into two category –one field work and second was laboratory work.

Field work includes conducting SPT ,Dynamic cone test, collection of disturbed as well as undisturbed soil samples from different location and different depth of sub-soil strata.

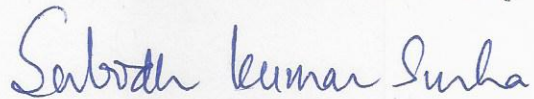
It was tried to get information from local people to get an idea about variation of water table during different season of year and also to get first hand information about type of foundation usually provided in the locality.

We thanks Prof. M.P.Jakhanwal (Retired) ,M.Tech ,Ph.D. ,Muzaffarpur Institute of Technology, Muzaffarpur for his

valuable advice during laboratory test and during preparation of report.

Client's help is gratefully acknowledged in providing Bore hole locations, cooperation and guidance during finalization of report.

We belief that the present report will serve the purpose, for which sub-soil investigation has been carried out.



SUBODH KUMAR SINHA  
Partner, Shamvwi Consultant

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# REPORT ON SUB-SOIL INVESTIGATION FOR THE CONSTRUCTION OF +2 SCHOOL AT H.S., KORARI, JODHAN BIGAHA, BELCHHI, PATNA

## 1. INTRODUCTION

The objective of subsoil investigation reported here in, were taken up, to find out the nature of subsoil at the site of the proposed construction and to recommend the type or types of foundation suitable for it and the corresponding allowable bearing capacity.

The necessary field tests were carried out at the site. Soil samples from various depths in the different bore holes were collected, transported, carefully to the laboratory and tested to determine the engineering properties of the soil.

Based on the test results, certain recommendation were made and given in this report, regarding the type of foundation suitable for the proposed project and the allowable bearing capacity for certain sizes thereof.

## 2. TOPOGRAPHY

The land in question was even.

## 3. FIELD WORK

The field work consists of boring, soil sampling and conduct of Standard penetration tests and Dynamic cone penetration tests.

### 3.1 BORING

An appropriate number of boreholes of adequate depth were sunk at suitable spots as per direction of Engineer-in-charge. The details of the boreholes are given in table-1.

Table 1: Details of bore holes

DIAMETER OF BORE MM	DEPTH M	BORE HOLE
150	10.5	3 Bore Holes (BH-1 to BH-3)

The borings were kept dry while advancing through partially saturated soil. The position of water table in a borehole was recorded at least 48 hours after the stopping of the boring operation.

For boring below ground water level, the borehole was kept filled with water upto that level during boring.

### 3.2 SAMPLING

Undisturbed & disturbed samples were collected at different depth/where change of strata occurred. Identification slips were provided both inside and outside the tube.

On arrival in laboratory, the identification slips were checked against the boring and sampling records. Samples were extracted from the tubes just before testing.

### 3.3 STANDARD PENETRATION TEST

This test was performed in the boreholes at interval of depth of 1.5m, or at the change of strata/ as per IS: 2131 of 1963.

### 3.4 DYNAMIC CONE PENETRATION TEST

This test was performed when a bore hole could not be advanced to desired depth due to caving- in of the soil, or when it was felt necessary to supplement the information gained from SPT. This test was performed, as per



## CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

relevant IS code till high value of penetration resistance was encountered or till desired depth of investigation was reached, at which stage the test was stopped.

### 4. LABORATORY TEST

Lab. Test was performed to determine the following properties of soil samples as per relevant I.S. code.

- (a) Natural moisture content.
- (b) Bulk density.
- (c) Atterberg's limits (on fine grained soil only)
- (d) Grain size analysis.
- (e) Specific gravity.
- (f) Shear test.
- (i) Unconfined/triaxial compression tests for fine-grained soils.
- (ii) Direct shear test for coarse-grained soils.
- (g) Consolidation tests for fine grained soils.
- (h) Organic content, chemical test etc.
- (i) pH of soil and water.
- (j) Free swell Index
- (k) Crushing strength test (uniaxial)

#### 4.1 SAMPLE EXTRACTION & PREPARATION OF TEST SPECIMENS

Samples for different tests were prepared as per method described in relevant IS code/as per method described in standard book.

#### 4.2 ROUTINE CLASSIFICATION TESTS.

Tests for the determination of natural moisture content, bulk density, Atterberg's limit, grain size distribution and specific gravity were performed as per IS code on representative disturbed soil samples, wherever felt necessary. The results were used in classifying the soils of different strata as per IS code 1498-1970.

### 5.0 PRESENTATION OF TEST RESULT

Results were presented in table form on the following pages.

#### 6.0 METHOD FOR CALCULATION OF ALLOWABLE BEARING CAPACITY

##### 6.1 COHESIVE SOIL

Net ultimate bearing capacity was calculated as per IS-6403-1981.  
 $q_d = cN_c S_c D_c I_c$

$q_d$  = net ultimate bearing capacity

$N_c = 5.14$

$S_c = 1$  for strip footing

$D_c = 1 + 0.2 * D/B$

$I_c = 1$  for vertical loading

$c$  = cohesion obtained through unconfined compression test for depth of  $2B/3$  below the foundation.

CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

Settlement criteria

$$S = H / (1 + e_0) * C_c * \log((p_0 + p_1) / p_0)$$

S= settlement

H=thickness of compressible layer

e<sub>0</sub>=initial void ratio

p<sub>0</sub>=initial effective pressure

p<sub>1</sub>=pressure increment

C<sub>c</sub>=compression index

6.2 Soil with the value of c & θ

Net ultimate bearing capacity was calculated as per IS 6403-1981

$$Q_d = c N_c S_c D_c I_c + q (N_q - 1) S_q D_q I_q + 0.5 R^* B N_r^* S_r^* D_r^* I_r^* w'$$

For local shear failure

$$\tan \theta' = 0.67 * \tan \theta$$

$$C' = 2 * c / 3$$

S<sub>c</sub>=S<sub>q</sub>=S<sub>r</sub>=1 for strip footing

$$D_c = 1 + 0.2 * (D/B) * \tan(45 + \theta/2)$$

I<sub>c</sub>=I<sub>q</sub>=I<sub>r</sub>=1 for vertical loading

$$D_q = D_r = 1 + 0.1 * (D/B) \tan(45 + \theta/2)$$

$$q = (R - R_w) * D$$

M= moisture content

R= bulk density of soil

R<sub>w</sub>=unit weight of water

L.L.= liquid limit

P.L.=plastic limit

S.L.= shrinkage limit

D=depth below ground level

Settlement criteria

The net allowable bearing capacity for a permissible settlement of 25mm, was obtained by

CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

teng's formula

$$Q_{na} = 3.5 \cdot (N-3) \cdot \left\{ \frac{(B+0.3)}{2} \cdot B \right\} \cdot \left\{ \frac{(B+0.3)}{2} \cdot B \right\} \cdot w' \cdot F_d$$

N= corrected N

$F_d = 1 + D/B$  less than or equal to 2

7.0 METHOD FOR CALCULATION OF CAPACITY OF CAST-IN-SITU PLANE PILE AS PER BIS 2911 Part I/Sec 2-1979

7.1 COHESIVE SOIL

Net ultimate bearing capacity of pile is given by :

$$Q = A_p \cdot N_c \cdot C_p + a \cdot C \cdot A_s$$

$A_p$  = cross sectional area of pile toe in cm<sup>2</sup>

$N_c$  = Bearing capacity factor usually taken as 9

$C_p$  = average cohesion at pile tip in Kg/cm

$a$  = reduction factor

$C$  = average cohesion throughout the length of pile in kg/cm<sup>2</sup>

$A_s$  = surface area of pile shaft in cm<sup>2</sup>

8.0 METHOD FOR CALCULATION OF CAPACITY OF CAST-IN-SITU PLANE PILE AS PER BIS 2911 Part III-1980

8.1 COHESIVE SOIL

Net ultimate bearing capacity of pile is given by :

$$Q = A_p \cdot N_c \cdot C_p + A_a \cdot N_c \cdot C'_a + C'_a \cdot A_s' + \alpha \cdot C_a \cdot A_s$$

$A_p$  = cross sectional area of pile toe in cm<sup>2</sup>

$N_c$  = Bearing capacity factor usually taken as 9

$C_p$  = cohesion of soil around toe.

$\alpha$  = reduction factor

$$A_a = \pi \cdot (D_u^2 - D^2) / 4$$

$C'_a$  = average cohesion around under ream

$D_u$  = dia of under-ream,  $D$  = dia of pile

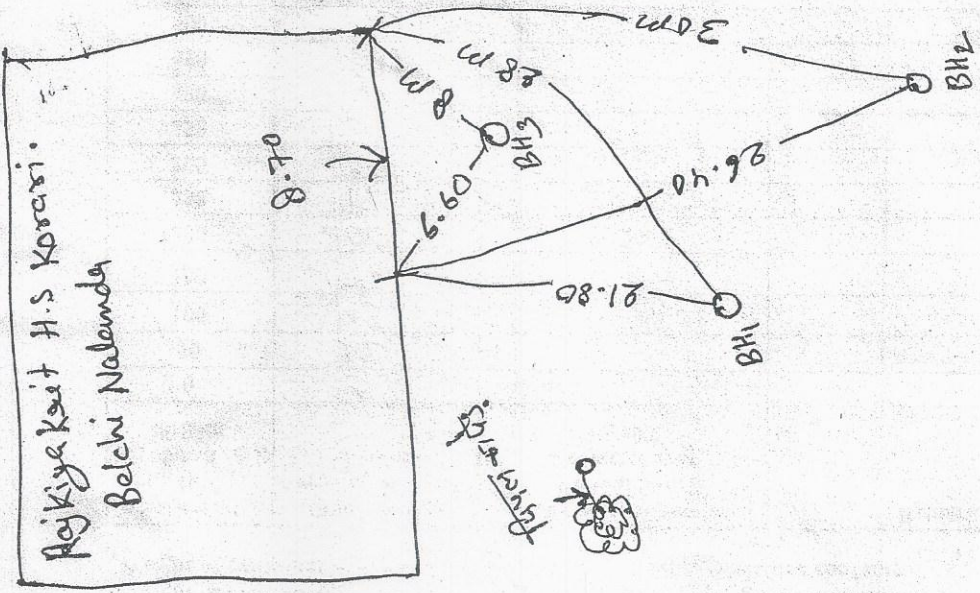
$A_s$  = surface area of pile shaft in cm<sup>2</sup>

$A_s'$  = surface area of stem

$A_s'$  = surface area of the cylinder circumscribing the under ream.



5/21



Rajkiya Kait H.S. Korari.  
Belchi Nalanda

Tajki Kait

School 95 Road

Road

Rajesh Kumar  
15/09/19  
JE/BSETR

4/2/21

अक्षय



SHAMVVI CONSULTANTS 414J.T.C.,FRASER ROAD, PATNA		NAME OF PROJECT : SOIL INVESTIGATION FOR CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA										BORING DATES		TERMINATION DEPTH :10.5M		TABLE NO :2										
SAMPLE NO	DEPTH OF SAMPLE	SPT BLOWS PER 30 CM		STANDARD PENETRATION RESISTANCE CURVE				GRAIN SIZE ANALYSIS				ATTERBERG'S LIMITS			DENSITY			SHEAR TEST			UNCONFINED COMPRESSION TEST					
		OBSERVED VALUE	CORRECTED VALUE	5	10	20	GRAVEL (%)	SAND (%)	SILT (%)	CLAY (%)	LIQUID LIMIT	PLASTIC LIMIT	PLASTICITY INDEX	BULK DENSITY (gm/cm <sup>3</sup> )	DRY DENSITY (gm/cm <sup>3</sup> )	NATURAL MOISTURE CONTENT (%)	SPECIFIC GRAVITY	TYPE OF TEST	COHESION c (kg/cm <sup>2</sup> )	ANGLE OF FRICTION IN DEGREE	VOID RATIO e <sub>0</sub>	COMPRESSION INDEX C <sub>c</sub>	UNCONFINED COMPRESSION TEST q <sub>u</sub> (kg/cm <sup>2</sup> )	COEFFICIENT OF VOLUME COMPRESSIONIBILITY M <sub>v</sub> (cm <sup>3</sup> /kg)		
DS	G.L.																									
UDS1																										
SPT1	1.5	10					0.1	6.20	93.7		41	22	19	1.99	1.68	18.2	2.61	UUT	0.4	19.0	0.81	0.12				
UDS2																										
SPT2	3	19					0.7	9.40	89.9		41	22	19	1.99	1.68	18.5	2.61	UUT								
UDS3																										
SPT3	4.5	30					1.7	31.90	66.4		35	24	11	1.98	1.65	19.8	2.60	UUT	0.26	23.00						
UDS4																										
SPT4	6	48					1.4	22.90	75.7		35	24	11	1.98	1.70	20.6	2.61									
UUT : UNCONSOLIDATED UNDRAINED TRIAXIAL SHEAR TEST		UCT : UNCONFINED COMPRESSION SHEAR TEST										DST : DIRECT SHEAR TEST														
I SAMPLE SLIPPED ~ TEST ON REMOULDED SAMPLE		UDS : UNDISTURBED SAMPLE										SPT : STANDARD PENETRATION TEST VALUE														
NOTES : CONSOLIDATION TEST RESULTS ARE FOR THE LOADING RANGE OF 5.0-10.0 t/m <sup>2</sup>																										









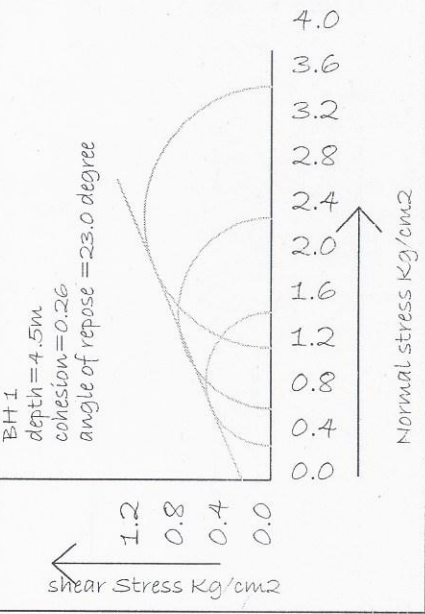
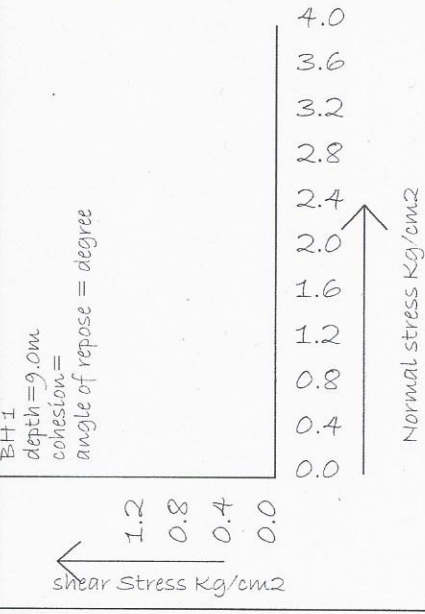
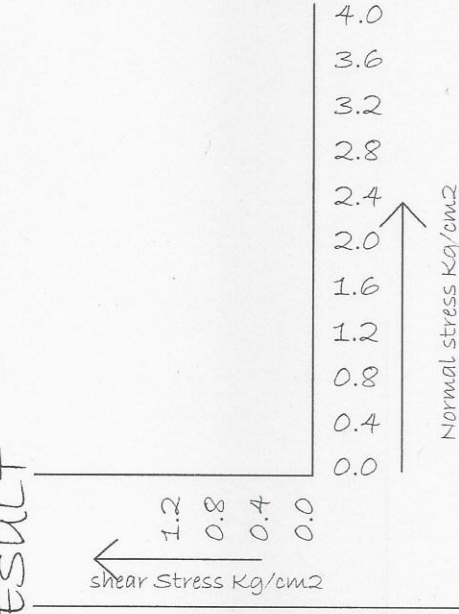
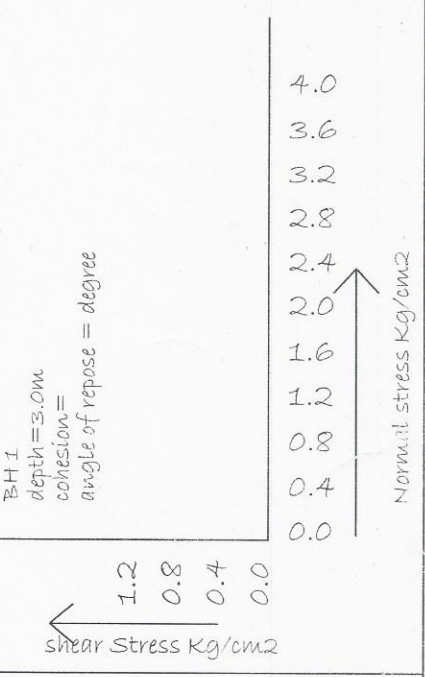
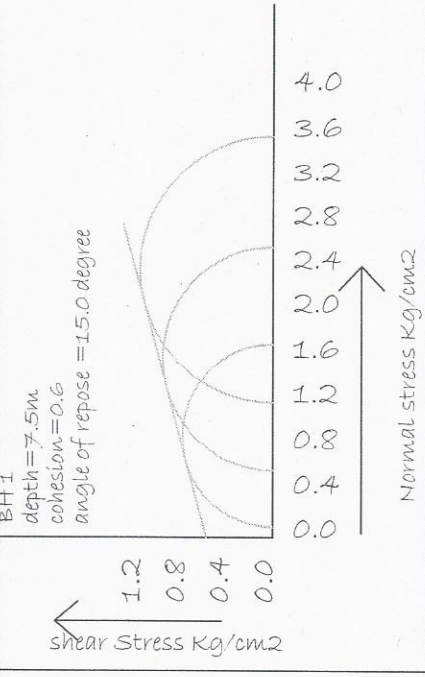
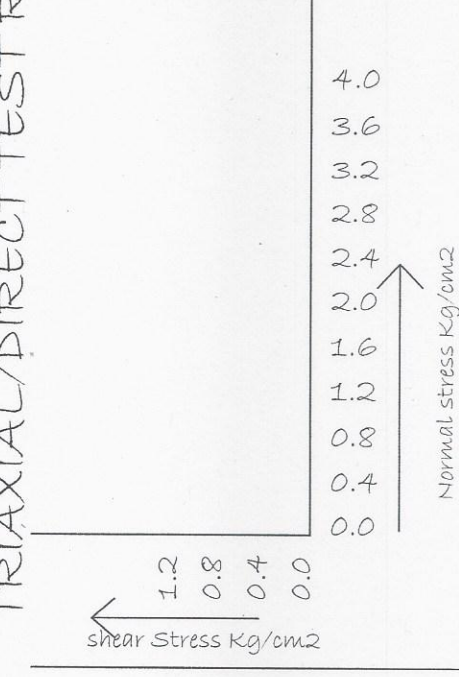
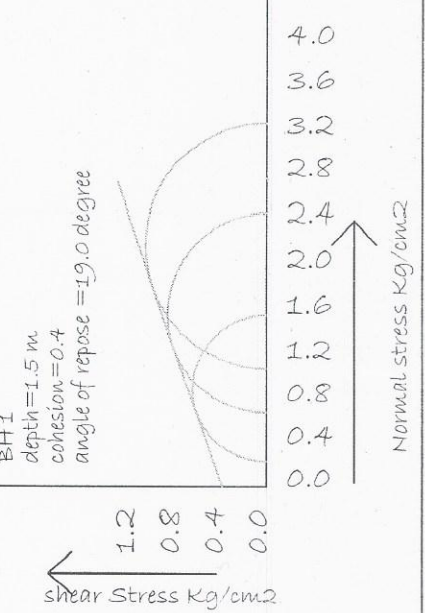
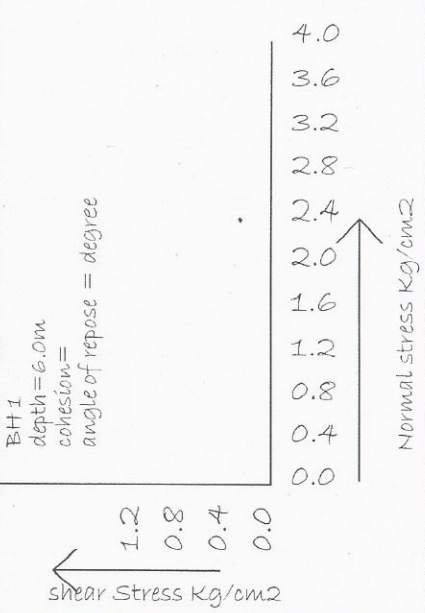
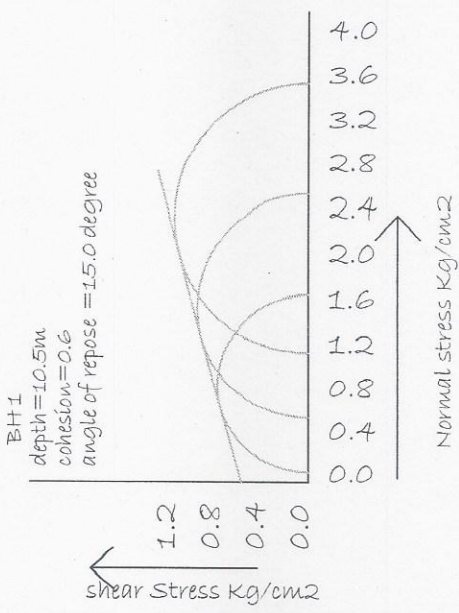






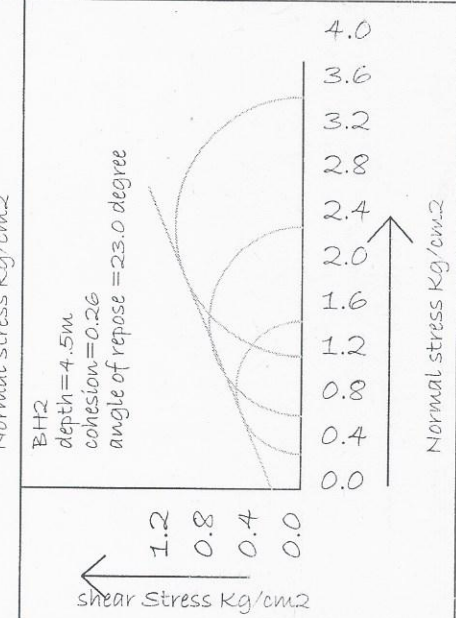
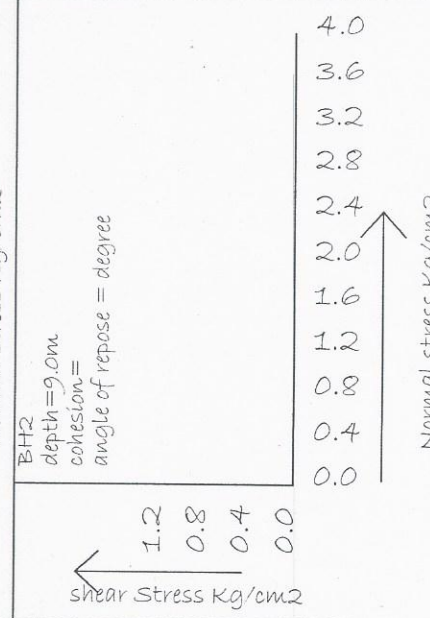
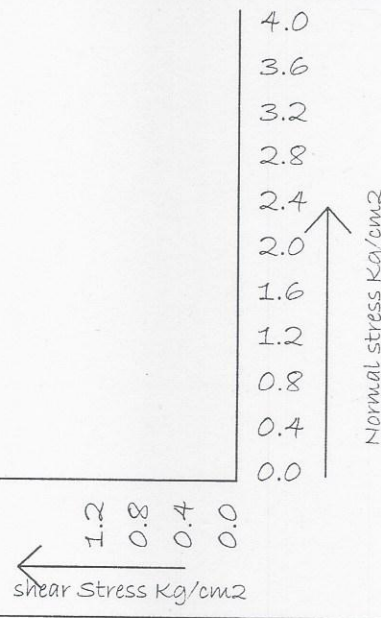
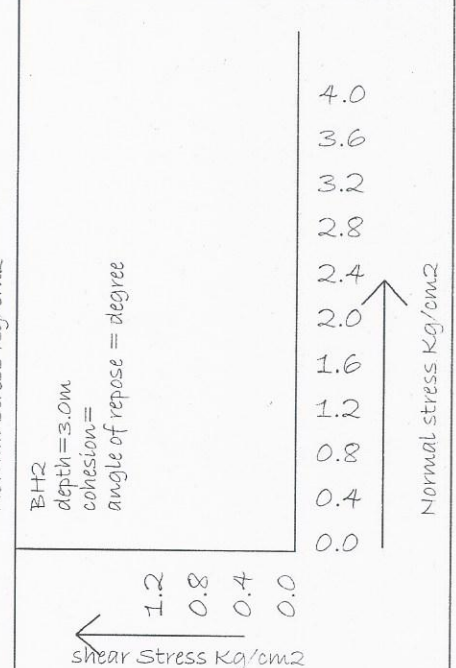
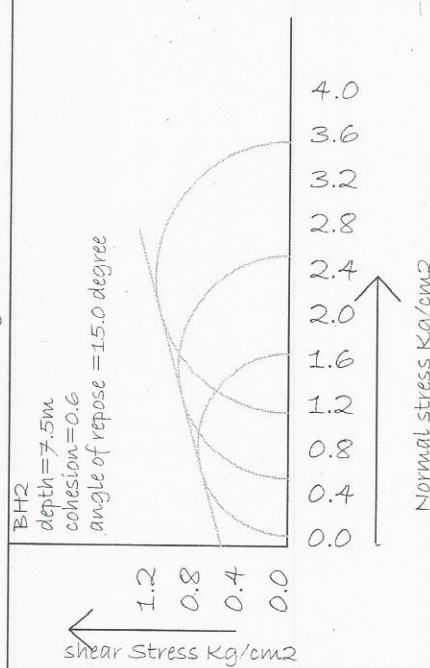
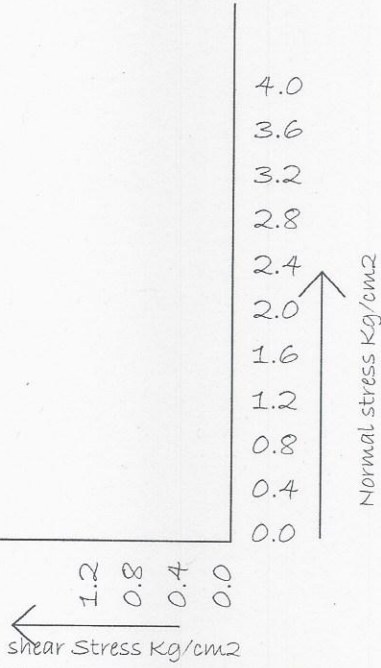
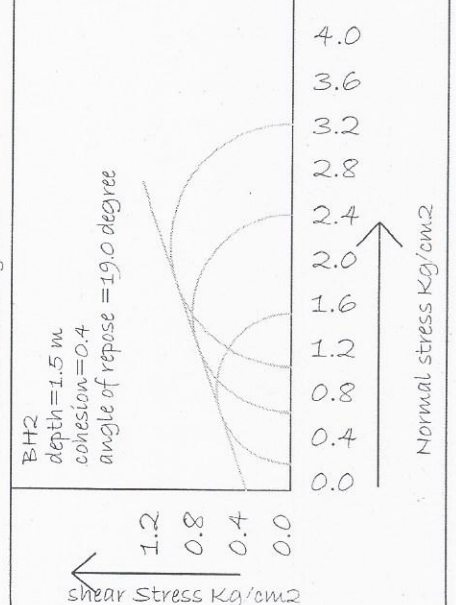
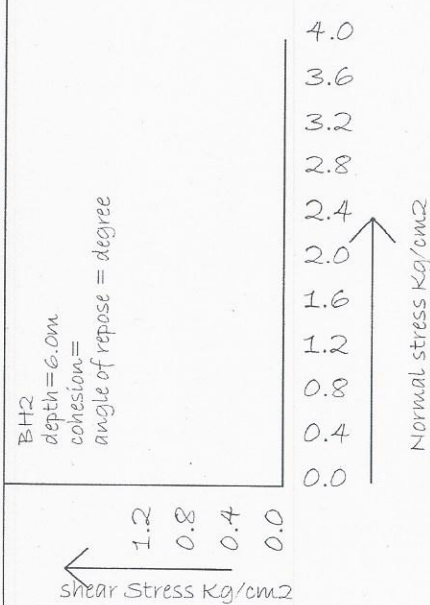
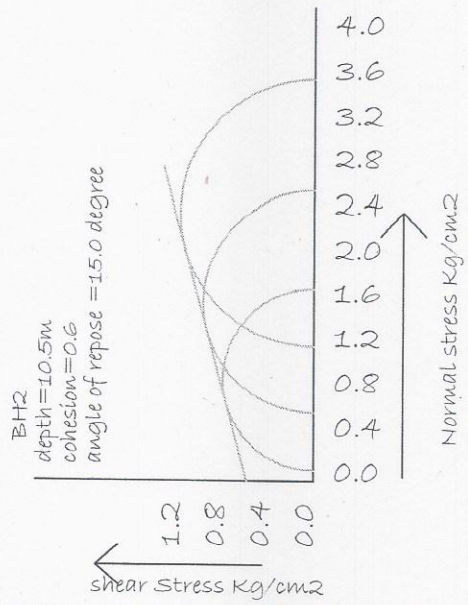


TRIAxIAL/DIRECT TEST RESULT



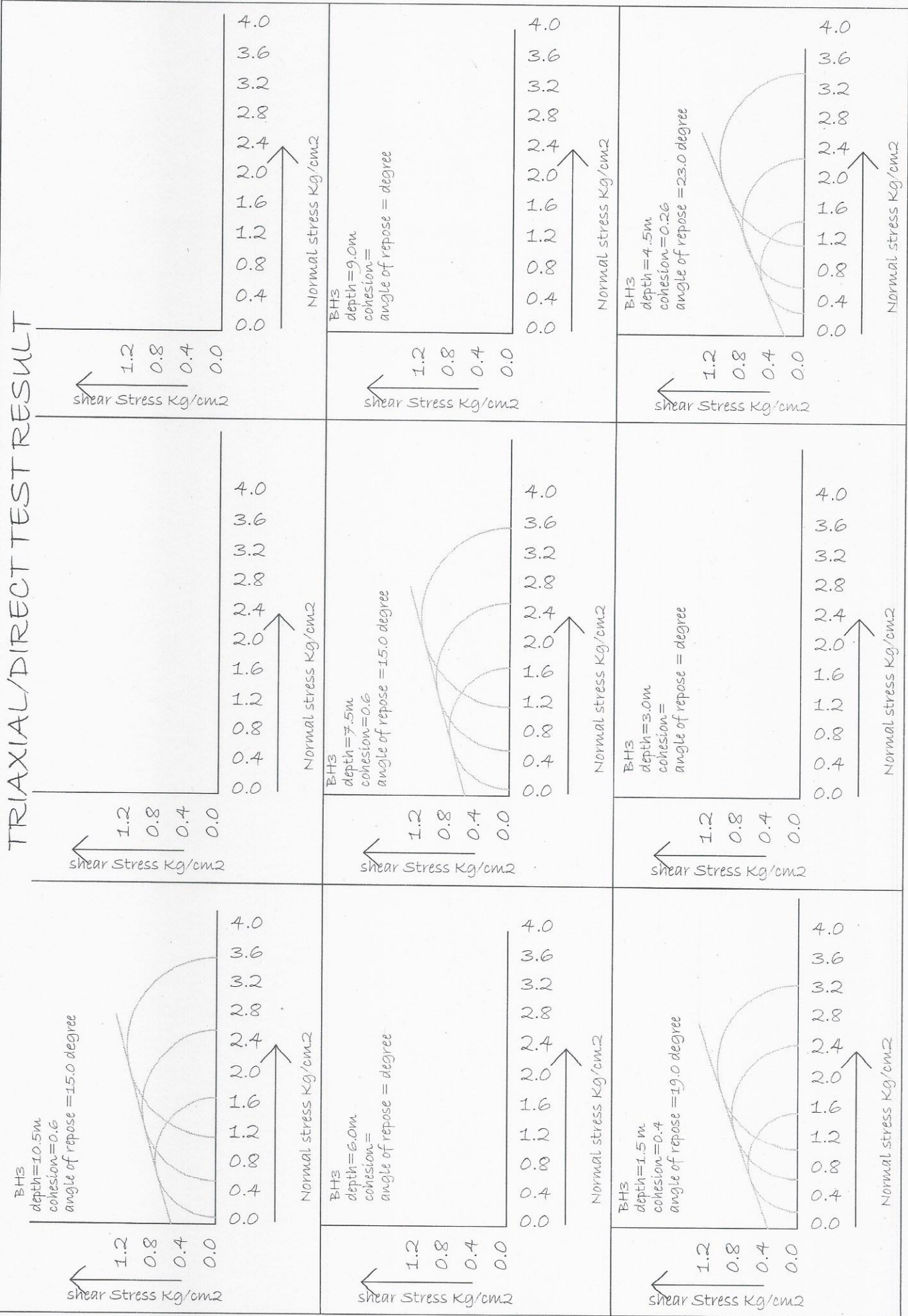


TRIAxIAL/DIRECT TEST RESULT



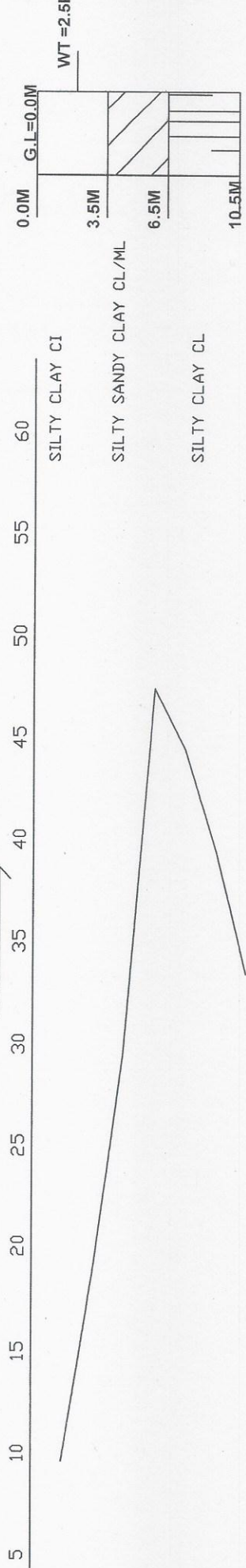


TRIAxIAL/DIRECT TEST RESULT



BORE LOG AND DEPTH ~ SPT GRAPH (CONSTRUCTION OF +2 SCHOOL AT H.S., KORARI, JODHAN BIGAHA, BELCHHI, PATNA

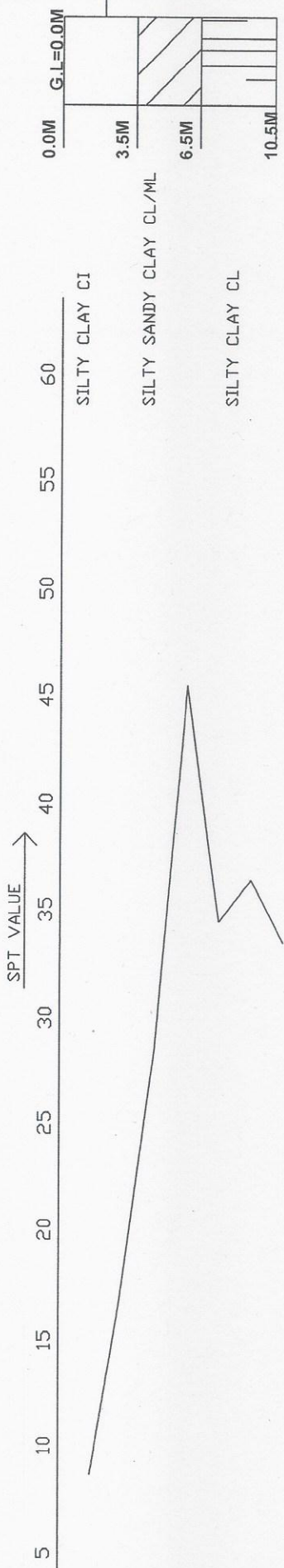
SPT VALUE →



BORE LOG

BH1

BORE LOG AND DEPTH ~ SPT GRAPH (CONSTRUCTION OF +2 SCHOOL AT H.S., KORARI, JODHAN BIGAHA, BELCHHI, PATNA)

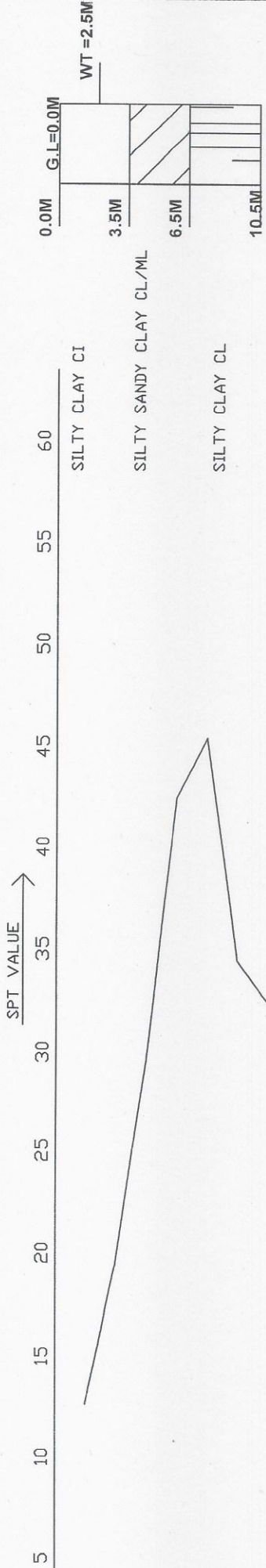


BORE LOG

BH2

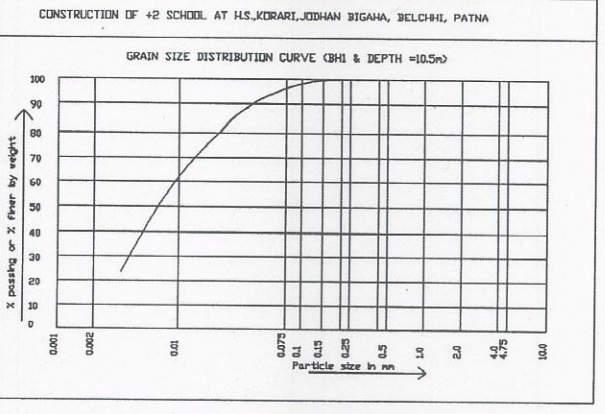
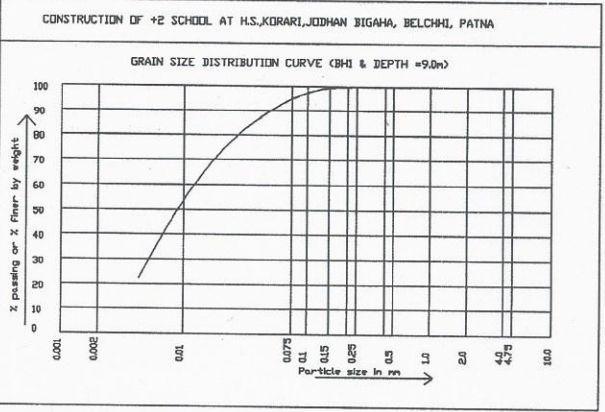
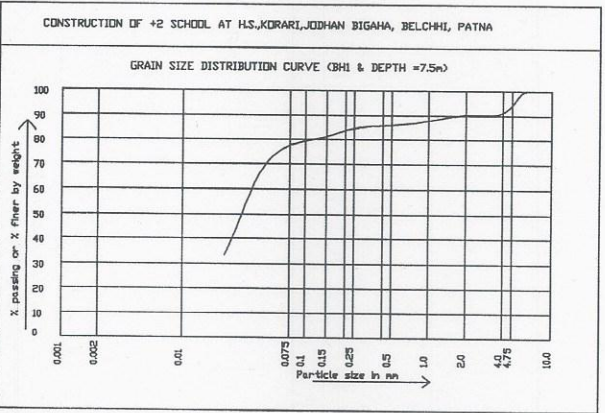
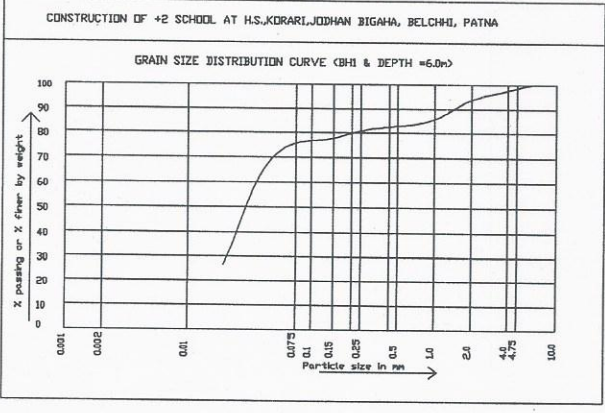
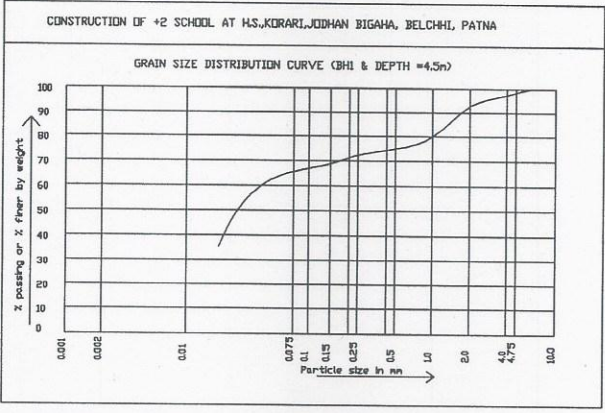
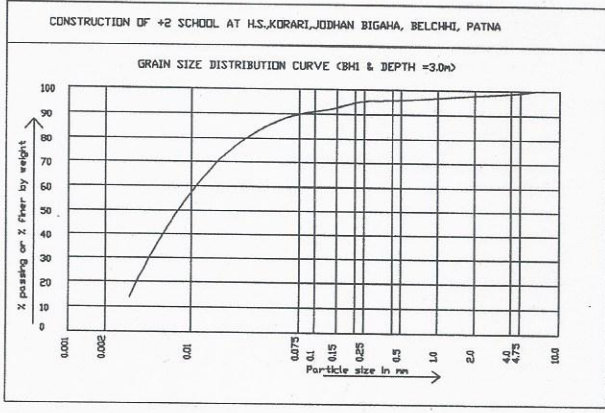
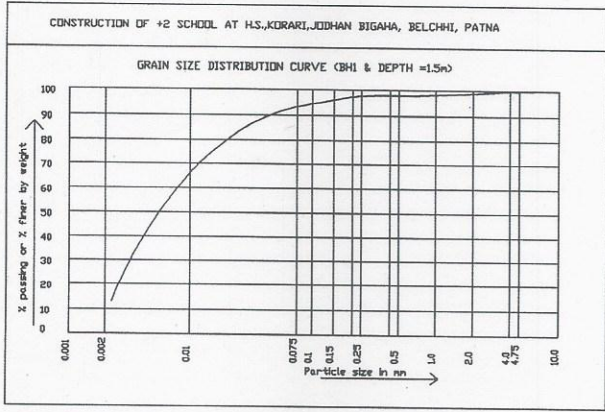


BORE LOG AND DEPTH ~ SPT GRAPH (CONSTRUCTION OF +2 SCHOOL AT H.S., KORARI, JODHAN BIGAHA, BELCHHI, PATNA



BORE LOG

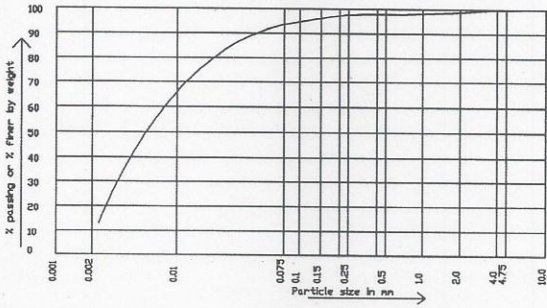
BH3





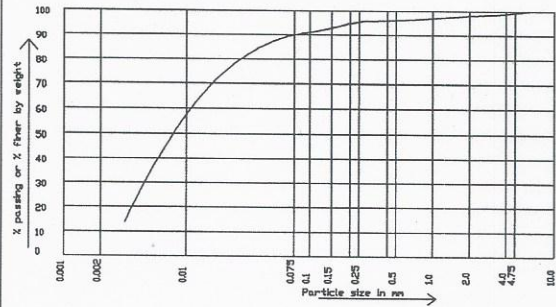
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH2 & DEPTH =1.5m)



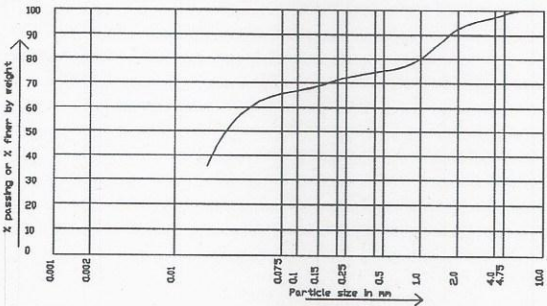
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH2 & DEPTH =3.0m)



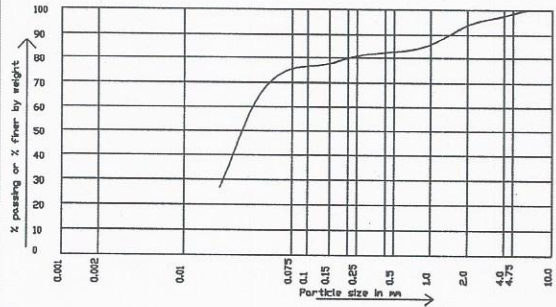
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH2 & DEPTH =4.5m)



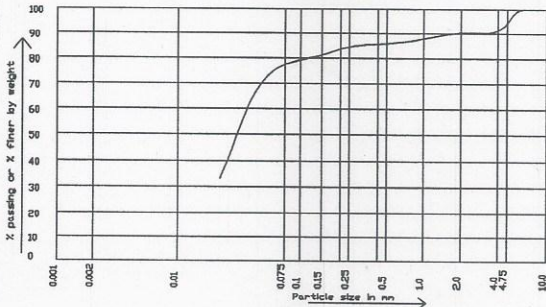
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH2 & DEPTH =6.0m)



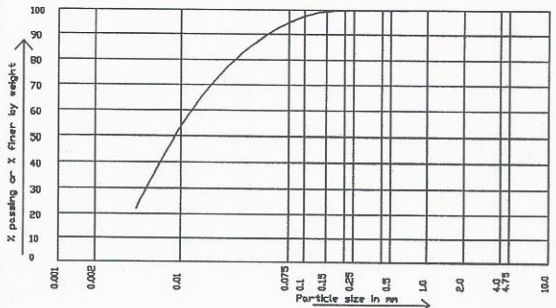
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH2 & DEPTH =7.5m)



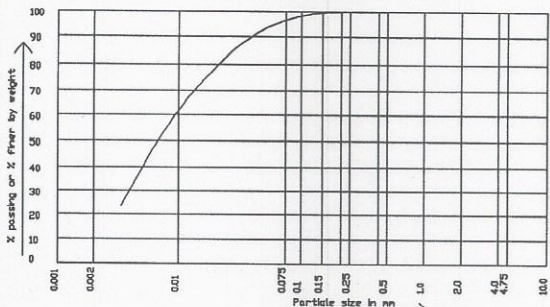
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH2 & DEPTH =9.0m)



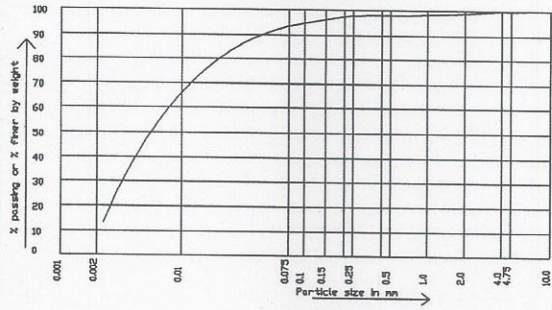
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH2 & DEPTH =10.5m)



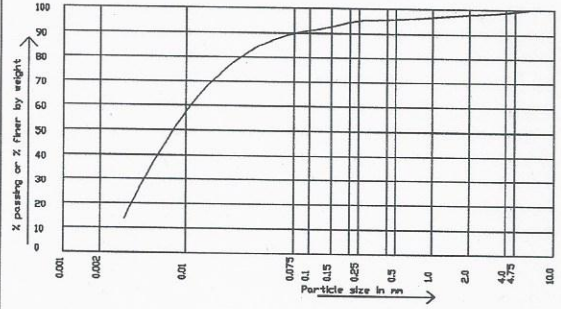
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH3 & DEPTH =1.5m)



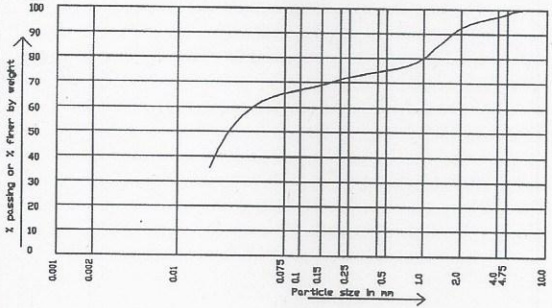
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH3 & DEPTH =3.0m)



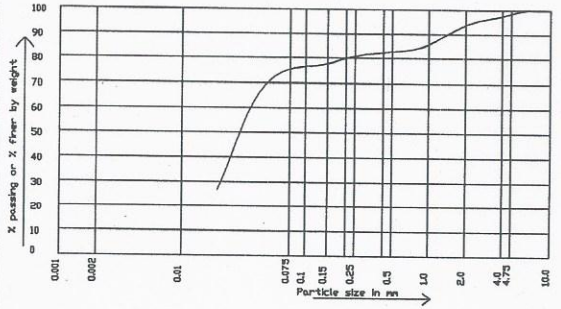
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH3 & DEPTH =4.5m)



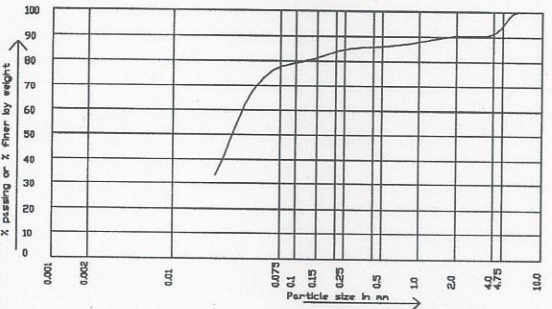
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH3 & DEPTH =6.0m)



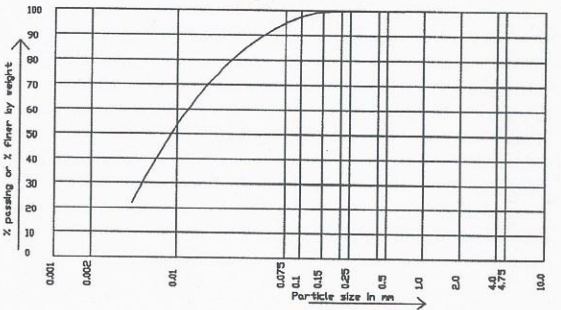
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH3 & DEPTH =7.5m)



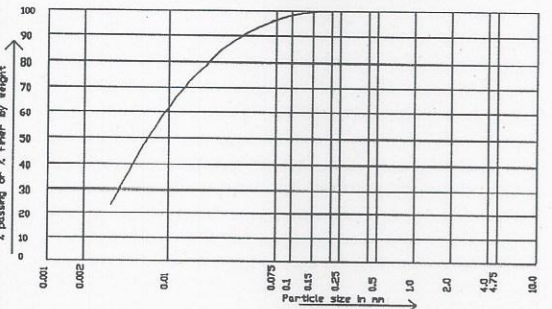
CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH3 & DEPTH =9.0m)



CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

GRAIN SIZE DISTRIBUTION CURVE (BH3 & DEPTH =10.5m)





NAME OF PROJECT : SOIL INVESTIGATION FOR CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA									
Calculation of settelement in clay for Strip Footing as per IS : 8009 (Part I)-1976 (Reaffirmed 1993)									
Width of FOOTING in meter	2.00		Assumption:Water table is at the foundation level.						
bearing capacity of soil in ton /m2 =	9								
Unit weight of soil in ton/m2 =	1.99								
Height of compressible soil in meter =H	3.00								
initial void ratio e0=	0.81								
Compression index Cc=	0.12	Assuming 2:1 pressure distribution							
Depth of Foundation in meter=	1.5								
Length of Footing=	2								
Determination of Bearing pressure at different depth below footing level factor for Strip footing									
Initial Effective stress at the top of clay layer=po				2.985	t/m2				
It is assumed that water table does not goes above footing level.									
Initial Effective stress at the bottom of clay layer=po				5.955	t/m2				
Average Effective stress on the clay stratum before construction=				4.47	t/m2		p0		
Additional Stress at the top of stratum due to construction=				9	t/m2				
Additional Stress at the center of stratum due to construction=9x2/(3/2+2)=				5.14	t/m2				
Average increase in stress after construction=				5.14	t/m2				
Hence Average effective stress on the clay stratum after construction=				4.47+5.14=	9.6	(p0+p1)			
Settelement s in mm =s=H/(1+e0)*Cc*Log10((po+p1)/p0)=0.12X3000X(LOG10(9.6/4.47))/(1+0.81)=					66.03	mm			
D/sqrt(L*B)	0.75								
Final D/sqrt(L*B)=	0.75								
L/B=	1.00								
Depth Factor=	1								
Correction for normally consolidated soil=	0.9								
Corrected Settelement s in mm=	59								
Rigidity factor=	1								
Corrected Settelement s in mm=	59								

SAMPLE CALCULATION OF CAPACITY OF UNDER REAM PILE for						NAME OF PROJECT : SOIL INVESTIGATION FOR CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA						
The load carrying capacity of the pile has been calculated using IS : 2911 (Part III) 1980, Clause 5.2.3.1												
These calculations are based on												
(a) in fine-grained soils, only on cohesion (c). In t/m <sup>2</sup> , taking angle of internal friction = 0												
This is likely to give the minimum capacity of the pile												
Pile diameter, D (m) =	0.3	Hence, area of pile base. Ap (m <sup>2</sup> ) =				0.071	& circumference (in m) of pile base j =					0.942
Under ream, diameter, Du (m) =	0.75	Hence, Aa (m <sup>2</sup> ) =		0.37	Spacing between under ream in m =		1.13	Hence, A's (m <sup>2</sup> ) =				2.66
The following values are taken in view of the codal provisions :						Surface area of pile's contact with soil, As (m <sup>2</sup> ) = j x t						
Reduction factor, α, depending on N.		0.5				where t = thickness of soil layer in contact with pile.						
Skin friction in clay, Qs = α * Ca * As.		Total Ultimate capacity of pile, Qu = Ap * Nc * Cp + Aa * Nc * C'a + C'a * A's										
Total Ultimate capacity of pile, Qu = Ap * Nc * Cp + Aa * Nc * C'a + C'a * A's + Qs						Nc =						9
Safe capacity of pile, Qsf = Qs / 3.0 + Qb / 3.0,												
takeing factor of safety =		3										
Depth of soil layer (m)	Soil type	Average cohesion Ca	cohesion cp t/m <sup>2</sup>	Thickness of layer, t [m]	Average cohesion C'a	As = m <sup>2</sup>	Ap * Nc * Cp I	Aa * Nc * C'a II	C'a * A's III	Qs = α * Ca * As IV	Ultimate capacity (TON)	Safe capacity (TON)
6	clay	3.2	2.6	6	4.2	4.59	1.66	13.99	11.17	7.34	34.16	11



Table 8

Soil stratification

DEPTH	SOIL TYPE	CONSISTANCY	CLASSIFICATION
0-3.5	SILTY CLAY	MEDIUM	CI
3.5-6.5	SILTY SANDY CLAY	MEDIUM TO STIFF	CL/ML
6.5-10.5	SILTY CLAY	MEDIUM TO STIFF	CL

WATER TABLE was found at 2.5m as reported in September'2019.

RECOMMENDATION

The present report is prepared on the basis of lab. Test result & field test conducted in the field. The lab. Test result is obtained by conducting different test on representative sample obtained through 3 no. of bore holes whose location and depth were decided by Engineer-in-charge of the department and shown in the bore hole location plan. These Boreholes are marked as BH1, BH2 and BH3.

The laboratory test of soil samples obtained in all bore holes are given in Tables 2-7. Study of these tables reveals :

- (a) Strata up to 10.5m consist of fine grained soil. Soil Strata from depth 3.5 to 6.5m consist of about 30% sand.

Both Shallow as well as pile foundation is feasible for the site. Bentonite or casing may be suggested to prevent the collapse of pile bore. Since, Permissible differential settlement depends on the structural parameters such as structural system, span etc., these can be obtained from the IS 1904, 1986.

By way of example the calculated value of safe capacity of certain type and size of Shallow foundation are being tabulated below: -

Shallow foundation

Depth below GL (m)	Width of foundation (m)	Allowable bearing capacity(t/m <sup>2</sup> )	Maximum expected settlement(mm)
1.5	2.0	9.0	60

CONSTRUCTION OF +2 SCHOOL AT H.S.,KORARI,JODHAN BIGAHA, BELCHHI, PATNA

By way of example the calculated value of safe capacity of certain diameter of piles using IS : 2911 (Part III) 1980: -

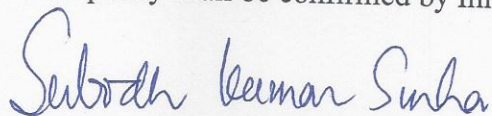
Double Under-reamed Pile Capacity

Depth of Pile below GL(m)	Dia of Pile (m)	Dia of Under-reamed (m)	Allowable Capacity (Ton)
6.0	0.3	0.75	11
8.0	0.3	0.75	15
8.0	0.4	1.0	24

Limitation

If the sub-soil condition is found much different from those reported here during trenching, suitable steps should be taken. Back filling over footing shall be done with proper compaction.

Pile capacity shall be confirmed by Initial and Routine pile load test as per relevant Indian codes.



SUBODH KUMAR SINHA  
Partner Shamvwi consultant